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Hydrological simulation uncertainties in small basins through the SWAT model

Incertezas na simulação hidrológica em pequenas bacias por meio do modelo SWAT

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RESUMO

A modelagem hidrológica é uma das principais ferramentas de apoio à gestão dos recursos hídricos. Entretanto, vários fatores dificultam a extrapolação dos parâmetros estimados em bacias com grandes áreas de contribuição para bacias de pequena dimensão. O objetivo deste trabalho foi analisar as incertezas na simulação de vazões em bacias de pequena dimensão. O processo metodológico envolveu a aplicação do modelo de simulação hidrológica Soil and Water Assessment Tool (SWAT) à bacia representativa do ribeirão Concórdia (30,74 km²) e à bacia do rio Itajaí (15.000 km²). Primeiramente, comparou-se 6 cenários distintos de discretização das unidades de respostas hidrológicas (HRUs) na calibração e validação da bacia hidrográfica do ribeirão Concórdia, avaliando-se a influência do número de HRU na simulação de pequena bacia hidrográfica. Em seguida, calibrou-se os parâmetros do modelo para a bacia do rio Itajaí, em 12 estações fluviométricas. Posteriormente, determinou-se uma tendência de variação dos parâmetros calibrados através da formulação de equações de regressão. Estas equações foram elaboradas a partir da correlação entre os índices físicos de cada estação fluviométrica considerada e seus valores. No presente estudo, a discretização das HRUs indicou que não há necessidade de aumentar o número de HRU quando o objetivo a ser alcançado é representar os picos do fluxo de água na pequena bacia hidrográfica. Os resultados obtidos com a aplicação das equações de regressão demonstraram que o coeficiente de compactidade (kC) pode gerar até 42,1% de variação na vazão média e 82,7% na Q₉₅ das sub-bacias. Portanto, a utilização das equações de regressão pode auxiliar na redução das incertezas geradas durante a calibração dos parâmetros.

Palavras-chave: Modelagem hidrológica; Escala espacial; Gestão de recursos hídricos.

ABSTRACT

Hydrological modelling is one of the main tools in water resources management application. However, several factors make it difficult to extrapolate the estimated parameters in basins with large contribution areas to smaller basins. This research analysed the uncertainties of flow simulations in small basins. The methodological process involves the application of the hydrological simulation model, the soil and water assessment tools (SWAT) to the basin of the Concordia River (30.74 km²) and the Itajaí River basin (15,000 km²). First, six different scenarios of the hydrological response units' (HRUs) discretisation were compared in the Concordia River basin calibration and validation, evaluating the number of HRUs influencing the small river basin simulation. Then, the model parameters for the Itajaí River basin were calibrated in 12 fluviometric stations. Subsequently, the calibrated parameter variation trend was determined through the formulation of regression equations. These equations were developed from the correlation among the physical indices of each fluviometric station and their values. In the present study, the HRUs' discretisation indicated there is no need to increase the number of HRUs when the goal is to represent the water flow peaks in the small river basin. The results obtained with the application of the regression equation showed the compactness coefficient (kC) could generate up to 42.1% of variation in the mean flow and 82.7% in Q₉₅ of the sub-basins. Therefore, the use of regression equations can help reduce uncertainties generated during parameter calibration.

Keywords: Hydrological modeling; Space scale; Water resources management.



INTRODUCTION

Water resources management is important for water quality control and availability. Unsustainable use can result in shortages and limitations in water consumption. Considering the qualitative and quantitative aspects of these resources, in an abundant situation water is treated as a free commodity. However, with the increase in demand conflicts can occur among the users (CARR; BLÖSCHL; LOUCKS, 2012; ARAÚJO; NASCIMENTO; OLIVEIRA, 2016).

To carry out water resources management it is necessary to understand the hydrological behaviour of the basin, making it possible to quantify the anthropic influence, effects and establish measures for its control (CARVALHO; CURI, 2016). The estimation of reference flow rates for the application of water resource management tools in small hydrographic basins is hampered by different factors, such as the low density of fluvimetric stations or short historical series. In these basins hydrological simulation models that can be used with an absence or scarcity of data could be a solution (ANDRADE; MELLO; BESKOW, 2013).

Distributed hydrological models with a physical or semi-physical base can be used to overcome these difficulties; they predict the existence of a relationship among the parameters and basin characteristics (DANIEL et al., 2011). However, several factors influence the hydrological model parameters values, such as the contributing basin size (SANTOS et al., 2014). This makes it difficult to extrapolate the contributing basin's estimated parameters to the small basins (LOEWEN; PINHEIRO, 2017). For the fluvimetric sections without data, the parameter estimation should happen based on the basin's physical characteristics, if the relationships are well established.

The soil and water assessment tool (SWAT) model (ARNOLD et al., 2012) has become an efficient tool in hydrological assessments of different basin scales and environmental conditions. Government agencies and private companies support decision-making associated with water resources using the SWAT. Also, universities use the tool in forecasting and controlling water quantity and quality. The SWAT model considers the digital elevation model of the region and automatically delimits the sub-basins. The river basin subdivision is displayed in hundreds to thousands of cells, representing homogeneous hydrological response units (HRUs) which reflect the differences in soil type, vegetation cover, topography and land use on site (ARAGÃO et al., 2013).

The SWAT model has shown satisfactory results in applications related to climatic and hydrological scenarios (BRESSIANI et al., 2015; FRANCESCONI et al., 2016; PEREIRA et al., 2016; MOLINA-NAVARRO et al., 2014), making it a potential tool to support water resources management. However, the studies that verified the HRUs and the sub-basins discretisation presented controversial results in relation to their applicability (HER et al., 2015; WHITE; CHAUBEY, 2005; BUENO et al., 2017). Bueno et al. (2017) found that the HRUs and the sub-basins discretisation are directly proportional to the improvement in the flow and water yield representation in the basin. In contrast, Her et al. (2015) concluded the main factors that require better HRUs discretisation are sediment production and water quality, rectifying the need to study the spatial discretisation influence on nutrient production raised by White and Chaubey (2005). They leave open the question

of how to formulate the model discretisation to obtain the best representation in small basins.

The sub-basins' discretisation has been considered in the calibration and validation of different hydrological models. On the European continent, Abbaspour et al. (2015) classified the study area (drainage area of 9.4 million km²) into 8,592 sub-basins, using 34 sub-basins for SWAT model calibration, in the nitrate flow simulation of the continent. In South America, Adam et al. (2015) applied the MGB-IPH model to the Parana basin (drainage area of 800,000 km²), to evaluate the effects of climate change on the flow. The model was calibrated and validated for four fluvimetric stations. The simulations for sub-basins with large drainage areas were performed. Sub-basins with small drainage areas were not evaluated.

Because it is a spatially distributed model, the SWAT model parameters represent the physical phenomena that occur in the basin. Consequently, they may be affected by the spatial scale change of the simulated drainage area. In the Itajai River basin, located in the Santa Catarina state, there is the Concordia River basin. It has a drainage area of 30.74 km², whose temporal data series is adequate for the application of the hydrological simulation model distributed of semi-physical base, allowing evaluation of the spatial discretisation effect on the reference flow estimation.

The objective of this research is to analyse the uncertainties in the SWAT parameter calibration in basins with different dimensions for further flow series spatialisation. The aim is to contribute to the advancement of water resources management in regions that lack data.

MATERIAL AND METHODS

The methodological process involves the application of the SWAT hydrological simulation model to the representative basin of the Concordia River, with a drainage area of 30.74 km², and to the Itajai River basin, with a drainage area of 15,000 km². Regarding the applications, the hydrological response units (HRUs) spatial discretisation and the model parameters' spatial distribution have been considered. The research consists of evaluating the HRUs' importance in the calibration and validation of a representative basin and determining a trend in the parameters adopted in the calibration of different river basin dimensions.

Study area

The Concordia River basin was chosen as the study area (Figure 1) due to its monitoring since 2005, initiated by the Environmental Recovery Project and the Small Producer Support Plan (PRAPEM/MICROBACIAS) and prepared by the Department of Agriculture and Rural Development of the state of Santa Catarina to improve the environmental management of rural areas.

The HRUs' discretisation was evaluated in the Concordia River watershed. This basin is comprised of rural activities with a predominance of small and medium producers, resulting in intense landscape fragmentation. The different land uses and occupations are relatively well distributed throughout this river basin (Figure 2).

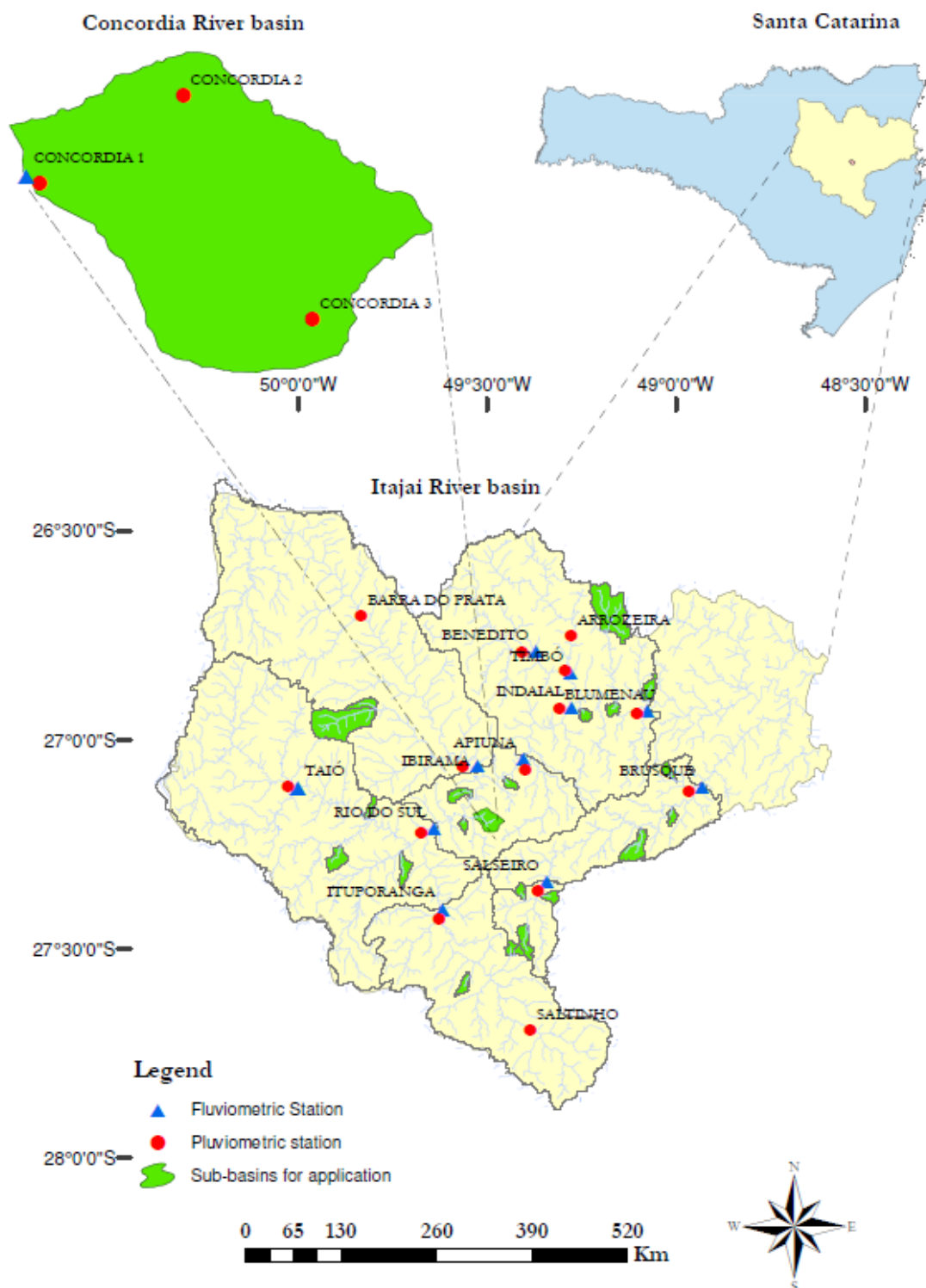


Figure 1. Study Area.
Source: Adapted from Santa Catarina (2007).

The choice of the Itajai River basin assisted in the project 'Effectiveness of the Framework in the Itajai River Basin', financed by the Sustainable Development Secretariat (SDS) of the state of Santa Catarina. The objective of this project is to carry out a water quality diagnosis of the Itajai River basin courses, aiming

at developing a framework. This study will contribute to the determination of nutrient loading in the river basin watercourses, making use of the flow series in each simulated HRU.

The spatial parameters were distributed by adjusting the SWAT model of the Itajai River basin in 12 fluviometric stations.

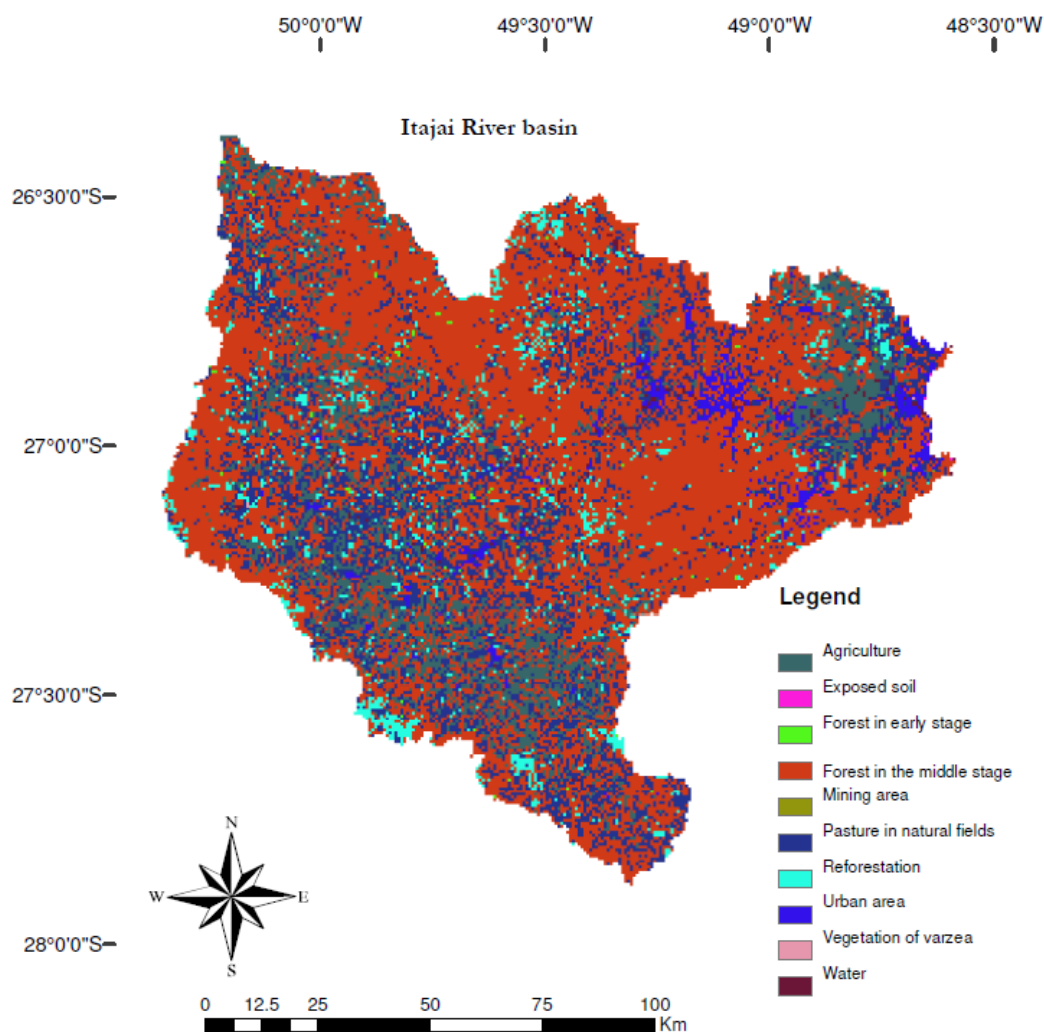


Figure 2. Distribution of land use in the Itajaí river basin.
Source: Adapted from Geoambiente Sensoriamento Remoto Ltda (2008).

The observed flow series were compared with the simulated flows series separately in each station. The criteria for choosing each fluviometric station was the monitored data quality and the drainage area variation in each sub-basin. Land use follows the same pattern as the Concordia River basin fragmentation, however there are places with high rates of human occupation.

Hydrological data

A total of 17 rainfall stations were used, of which 14 were available in the HidroWeb database of the National Water Agency and the others obtained through monitoring in the Concordia River basin. The daily data available for each rainfall station covers the period from 01/01/2002 to 12/31/2016. The failures used a simple linear regression method (ANA, 2012b). In this method, the failed rainfall station precipitation is always correlated to its neighbouring station.

The first step was to verify the influence of the HRUs' discretisation in the small Concordia River basin using the fluviometric station. The calibration took place during the period 01/01/2008 to 12/31/2013. Over the first two years, the data were left in order to heat the model. Model validation occurred during the period 01/01/2014 to 12/31/2016. Both periods mentioned are available in daily fluviometric data form.

The spatial discretisation effect performed in the Itajaí River basin used 12 fluviometric stations (Table 1) for calibration of the model parameters. Correlations with the drainage areas' physical factors were also analysed. The periods considered for calibration and validation occurred from 01/01/2008 to 12/31/2012 and from 01/01/2013 to 12/31/2016, respectively. The model-heat period was the same as the previous stage (influence of the HRUs' discretisation in the small Concordia River basin). The fluviometric data series were consistent with the methodology described by ANA (2012a).

Table 1. Summary of physical characteristics in the fluviometric sections.

Fluviometric Station	Area km ²	Perimeter Km	kC	KF
Blumenau	11,803	682.88	1.771	0.705
Indaial	11,265	659.44	1.740	0.869
Apiuna	9,070	591.40	1.739	0.773
Rio do Sul	5,160	465.50	1.814	0.805
Ibirama	3,330	312.63	1.517	0.336
Ituporanga	1,650	269.69	1.859	0.434
Timbo	1,600	199.47	1.396	0.902
Taio	1,570	199.88	1.412	0.979
Brusque	1,240	239.73	1.906	0.285
Benedito	717	152.36	1.593	0.651
Salseiro	286	98.98	1.639	0.393
Concordia	30	25.24	1.291	0.403

Table 2. SWAT parameters used for calibration.

Parameter	Description	Interval
v_ALPHA_BF	Base flow recession coefficient	0 - 1
v_GW_DELAY	Time interval for aquifer recharge	0 - 500
v_GWQMN	Water limit for base flow occurrence	0 - 5,000
v_OV_N	Manning number for terrestrial flow	0.01 - 30
v_EPCO	Water absorption coefficient by plants	0 - 1
v_ESCO	Soil evaporation compensation coefficient	0 - 1
v_SURLAG	Surface runoff delay coefficient	1 - 24
v_CH_K1	Surface runoff loss transmission coefficient	0 - 300
v_CH_N1	Manning Number for tributaries watercourses	0.01 - 30
v_GW_REVAP	Saturated zone water rise coefficient	0.02 - 0.2

Table 3. Parameters used for the model calibration in the sub-basins.

Parameter	Description	Interval
v_ALPHA_BF	Base flow recession coefficient	0 - 1
v_GW_DELAY	Time interval for aquifer recharge	0 - 500
v_GWQMN	Water limit for base flow occurrence	0 - 5000
v_OV_N	Manning number for terrestrial flow	0.01 - 30
v_EPCO	Water absorption coefficient by plants	0 - 1
v_ESCO	Soil evaporation compensation coefficient	0 - 1
v_SURLAG	Superficial runoff delay coefficient	1 - 24
v_CH_K1	Surface runoff loss transmission coefficient	0 - 300
v_CH_N1	Manning Number for tributaries watercourses	0.01 - 30

Table 4. Summary of HRUs discretizations.

HRUs discretization	Area (ha)	Sub-basins	HRU	Use (%)	Soil (%)	Declivity (%)	Land use, soil and slope
1590	15	127	1590	5	90	5	No
780	15	127	780	15	70	15	No
387	15	127	387	70	15	15	No
127	15	127	127	-	-	-	Yes
64	60	21	64	65	15	20	No
21	60	21	21	-	-	-	Yes

The SWAT model

The SWAT model interface, the ArcSWAT, was used as a tool to analyse the steps discussed in the paper: the HRUs' discretisation uncertainties (Table 2) and the spatial distribution effect of the parameters (Table 3).

In the parameters' spatial distribution step, the value 400 ha was used to discretize the Itajai River basin into 2,086 sub-basins. Each HRU was considered equal to a sub-basin. Thus, 2,086 HRUs were employed to simulate the hydrological processes in the Itajai River basin.

The computer applied in the project development was a laptop with a 64-bit operating system, 4 CPUs, 16 GB RAM and 2.8 GHz processors. Processing time ranged from 1 hour to 24 hours depending on the number of HRUs used in the model discretisation.

Calibration and validation

Calibration occurred in an automated manner using the software SWAT-CUP (version 5.1.6, Texas A&M University) with the SUFI-2 algorithm (ABBASPOUR, 2015). For each algorithm interaction 1,000 simulations were adopted. The objective functions which indicated the SWAT adjustment were the Nash-Sutcliffe coefficient (NS) and the bias percentage coefficient (Pbias).

The NS was selected because it indicates how close the simulated data is to the observed data. Pbias examines whether the observed data sum is greater or less than the simulated data sum. Positive Pbias values indicate the model is underestimating the observed series. Negative Pbias values indicate the model is overestimating the observed series.

As the SWAT model was evaluated on a daily basis, the satisfactory values range recommended by Moriasi et al. (2007) for the NS and Pbias coefficients were adopted, being: $NS \geq 0.5$ and $Pbias \leq \pm 25\%$.

HRUs Discretization

The influence of the number of HRUs observed on the model representation was determined based on six distinct discretisations (Table 4) of the Concordia River basin. In these, the land use importance, soil type and slope varied, as well as the number of sub-basins. The mean area value for sub-basin determination indicates the number of HRUs the model can

generate. It was decided to assign an average area with 15 ha initially, producing 127 sub-basins. In order to achieve greater variation in spatial discretisation, it was necessary to increase the average area to 60 ha, reducing the total sub-basins to 21.

The parameters were modified over the prefix 'v' (substitute) within the values range represented in Table 2. The same pattern for each scenario was considered.

At the end of calibration and validation the NS and Pbias coefficients, the processing time and the P-values of each scenario were compared to verify patterns in the model response variations.

Parameters spatial distribution

The model parameters' spatial distribution was performed by comparing the parameter values of each Itajai River basin fluvimetric station with the station's physical indexes. In this way, the model for each section displayed in Table 1 was calibrated and validated. The indices surveyed were: the area, the perimeter, the compactness coefficient (kC) and the form factor (kF).

The kC associates the basin shape with that of a circle through the relationship between the basin perimeter and the circle perimeter of the same basin area. The calculated value tends to the unity while the basin shape approximates that of a circle (TONELLO et al., 2006). Equation 1 was used to calculate kC:

$$kC = 0.28x \frac{P}{\sqrt{A}} \quad (1)$$

where: P is the perimeter in km; A is the area in km².

The kF is the mean width ratio to the basin axis length (from the mouth to the furthest point in the area), comparing the basin aspect with a rectangle (TONELLO et al., 2006). The kF was calculated according to equation 2:

$$kF = \frac{A}{C^2} \quad (2)$$

where: A is the basin area in km²; C is the length in KM from the mouth to the furthest point in the area.

The area and perimeter were determined using ArcGIS software (version 10.1, Esri), based on the hydrology maps and hydrographic units available on the Digital Maps platform in the state of Santa Catarina with a 1:50,000 scale.

The Itajai River basin was calibrated at each fluvimetric station, following the order upstream to downstream. To perform this calibration the HRUs corresponding to the fluvimetric station in question were selected and the SWAT model parameters modified, generating improvements of the Nash and Pbias performance coefficients.

After calibrating and validating the model, Statistica (version 10, StatSoft) software was used to correlate the physical indices with the weighted parameters for each basin, using the Pearson correlation coefficient. Regression equations were also generated for each parameter.

The Pearson correlation coefficient is a statistic that represents the variation index between two factors. In stronger correlations, the coefficient will tend to values closer to ± 1 . Positive values indicate a direct correlation and negative values

the inverse correlation (MUKAKA, 2012). The value considered acceptable was proposed by Mukaka (2012), which indicates the moderate correlation should be $\geq |0.5|$.

Finally, to apply the regression equations and determine the mean deviation between the simulations, 22 sub-basins with a drainage area ranging from 9 to 110 km² were randomly selected. The deviation was estimated considering the simulated flows series with the calibrated parameters for the Itajai River basin and the parameters in the small basin area, estimated according to their physical characteristics. It was decided to verify the average flow and the flow with 95% occurrence frequency (Q95), as these are important indicators for water resources management.

RESULTS AND DISCUSSION

The Concordia River basin calibration and validation resulted in the hydrographs shown in Figure 3. These were compared to six different HRU discretisations of flow values observed in the basin. The different discretisations showed similar behaviour over time and that the increase in the number of HRU caused a slight variation in water flow. However, as found by Her et al. (2015), the HRUs discretisation process has a strong effect on the ground, land use and soil composition, so that the HRUs' low number compromises sediment production and water quality.

The NS coefficient showed reduced variation with the increase in the number of HRUs, demonstrating that the number of sub-basins is more relevant than the HRUs' discretisation. This result corroborates the hypothesis of Bueno et al. (2017), who found the number of sub-basins tended to cause greater sensitivity in the NS coefficient.

The coefficient Pbias improved according to the increase in the number of HRUs. The more the sub-basin is discretized the better the model represents water storage. However, processing time is a limiting factor in the model calibration, which tends to increase significantly with the number of HRUs, as shown in Table 5.

The P-value for each parameter tests the null hypothesis that the coefficient is equal to zero. In this research, the P-value must be less than 0.05 to reject the null hypothesis. An indicator with a low P-value is likely to be a significant addition to the model, because changes in the indicator value are related to changes in the response variable (ABBASPOUR, 2015).

In this context, when the number of HRUs is less than 400, the parameters influenced are Alpha_bf, Gw_Delay, Gwqmn, Ch_k1 and Ov_n (Figure 4). As for the discretisations that have a greater number of HRUs, the parameters Gw_Delay, Gwqmn and Ov_N continued to be representative. The parameters Esco, Surlag and Gw_Revap gain importance in the calibration. This shows that by increasing the number of HRUs the model better represents the base flow, requiring coefficients to compensate the flow in a saturated state and during soil evaporation.

Therefore, when the objective is to represent the flow peaks in the basins, the HRUs' discretisation does not become a limiting factor. However, if the objective is to verify the base flow in the small basin simulation, the number of HRUs is fundamental for better representation of the hydrological processes.

Hydrographs of the fluvimetric sections examined for the Itajai River basin calibration and validation (Figure 5) indicate

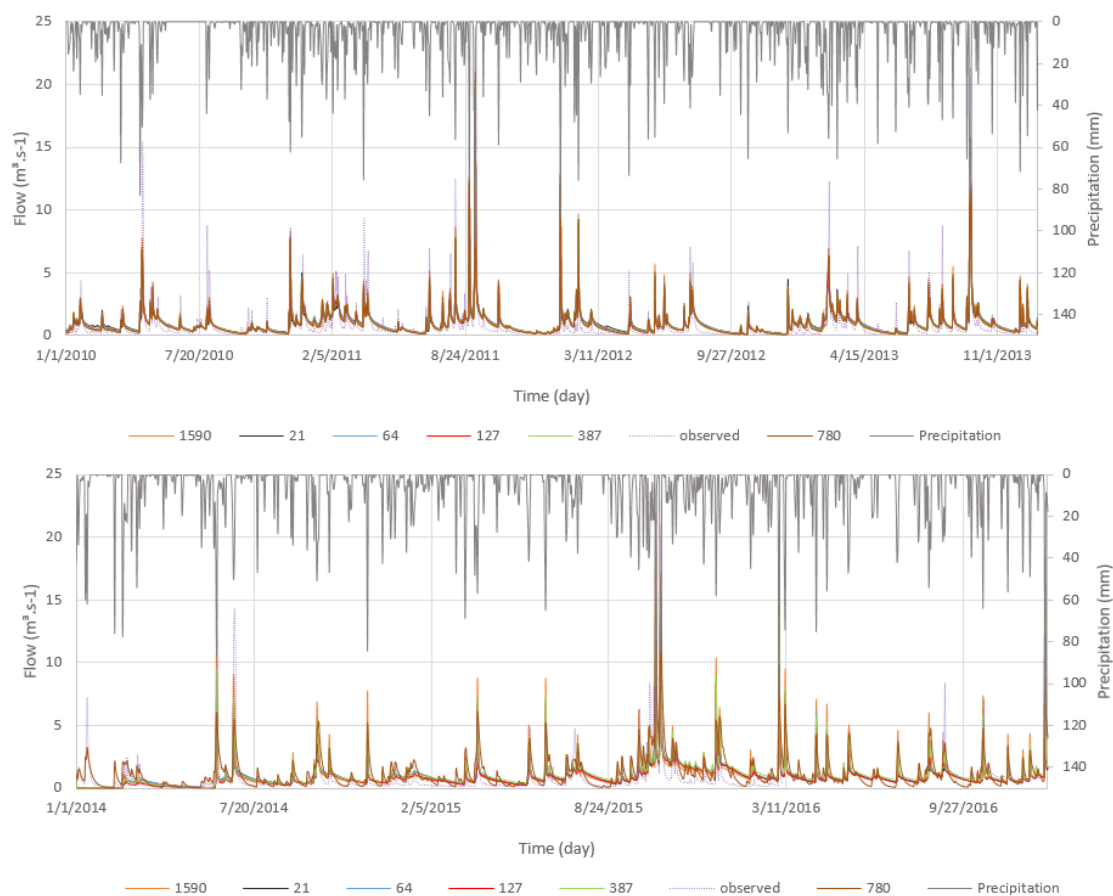


Figure 3. Calibration hydrograms (2010 to 2013) and validation (2014 to 2016) of the different HRUs discretizations.

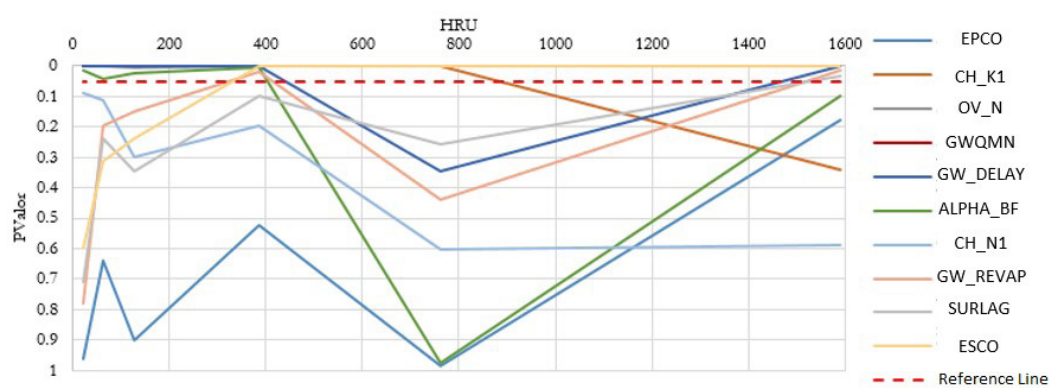


Figure 4. P-value result obtained by calibration of HRUs in Swat-Cup.

Table 5. NS, Pbias (%) and processing time for discrete HRUs.

HRUs	Calibration		Validation		Processing time (h)
	NS	P _{bias}	NS	P _{bias}	
1590	0.65	-10	0.52	-17.8	24:15:30
780	0.68	-12.5	0.58	-22.5	12:30:25
387	0.64	-16.1	0.57	-16.5	9:53:28
127	0.65	-16.1	0.53	-17.2	6:29:29
64	0.61	-16.3	0.56	-17.4	1:48:54
21	0.63	-15.9	0.57	-18.3	1:06:12

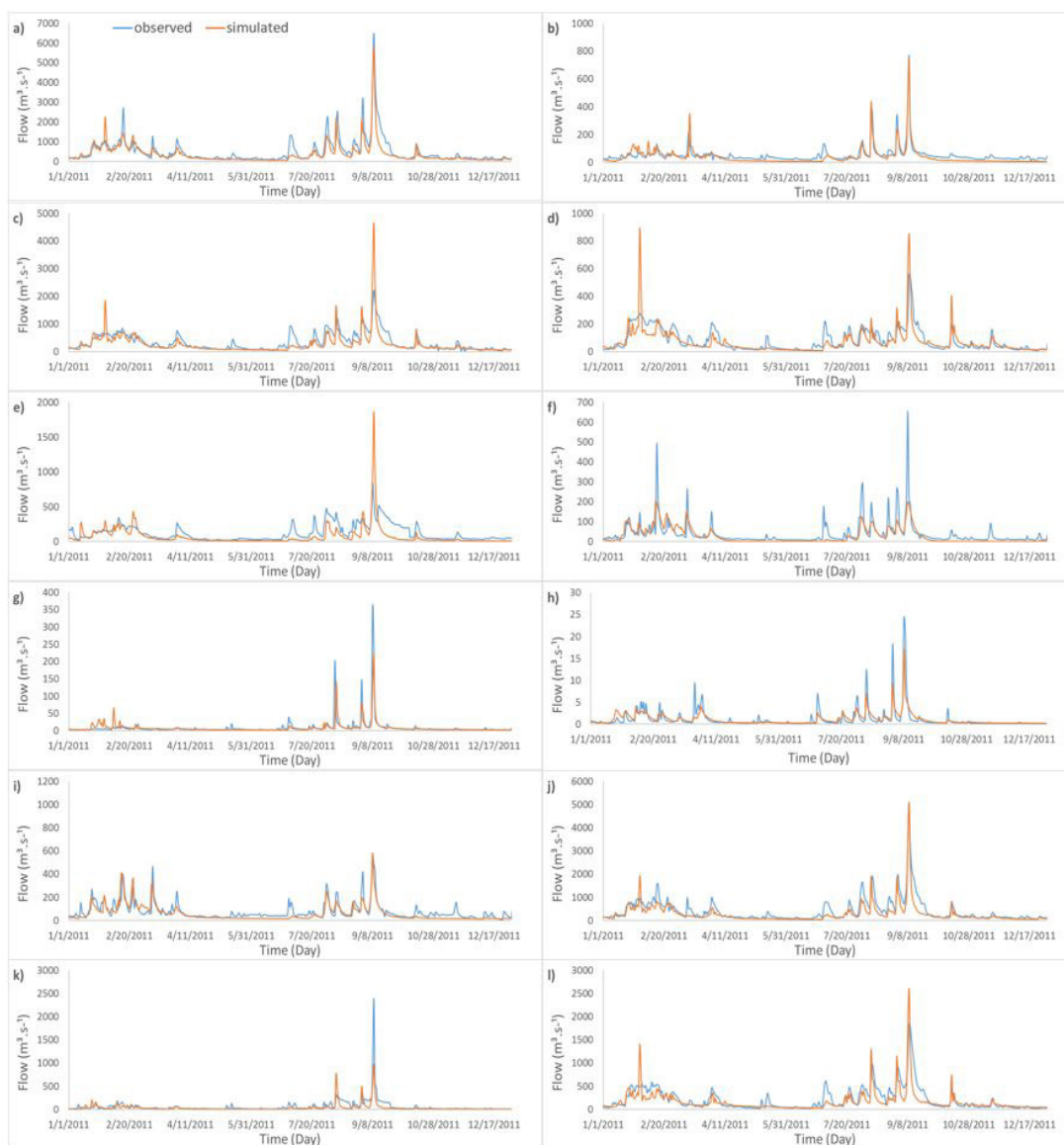


Figure 5. Hydrographic calibration charts for the year 2011 referring to the points of a) Blumenau, b) Brusque, c) Apiuna, d) Taio, e) Ibirama, f) Benedito, g) Salseiro, h) Concordia, i) Timbo, j) Indaial, k) Ituporanga and l) Rio do Sul.

the accuracy of the SWAT model in representing the parameters' spatial distribution. The hydrographs presented reduced variation in the maximum flow peaks.

The NS and Pbias coefficient values calculated for the SWAT model calibration and validation in the sub-basins' discretisation stage are presented in Table 6. The results show there are differences in the model parameters' spatial representation, which can be observed in the coefficient variation.

NS could be related to the precipitation spatial distribution that occurred in the basin, indicating that even when the model is fed by several rainfall stations it has difficulty reproducing high flow in some fluviometric stations, corroborating Pereira et al. (2014) and Zhang, Xu and Fu (2014), who obtained similar results.

The Pbias variation could be related to the soil discretisation, which causes low water storage. Consequently, the model estimates a

smaller flow than is observed. This effect becomes more significant when verifying the validation. According to Pfannerstill, Guse and Fohrer (2014), the SWAT model tends to generate more uncertainties in low flow periods. However, the coefficients presented satisfactory values, demonstrating the model's ability to simulate flow in the hydrographic basin.

The correlation among the parameters and physical indexes was obtained only with kC (Figure 6). This was the only factor that presented correlation values above 0.5, evidence of a moderate correlation.

The parameters Surlag and Ch_K1 had the highest correlation values. The Surlag parameter is a delay coefficient in the surface runoff, directly related to the daily surface runoff amount that discharges into the main channel (NEITSCH et al.,

Table 6. NS and Pbias results spatial distribution.

Fluviometric Station	Calibration (2010 - 2012)		Validation (2013 - 2016)	
	NS	P _{bias} (%)	NS	P _{bias} (%)
Blumenau	0.74	23	0.71	25
Indaial	0.81	13	0.74	25
Apiuna	0.54	23	0.58	25
Rio do Sul	0.65	19	0.61	25
Ibirama	0.52	24	0.71	25
Ituporanga	0.60	22	0.5	23
Timbo	0.50	16	0.64	25
Taio	0.55	17	0.5	24
Brusque	0.66	20	0.67	18
Benedito	0.51	22	0.52	25
Salseiro	0.50	17	0.57	18
Concordia	0.67	-5	0.69	-20

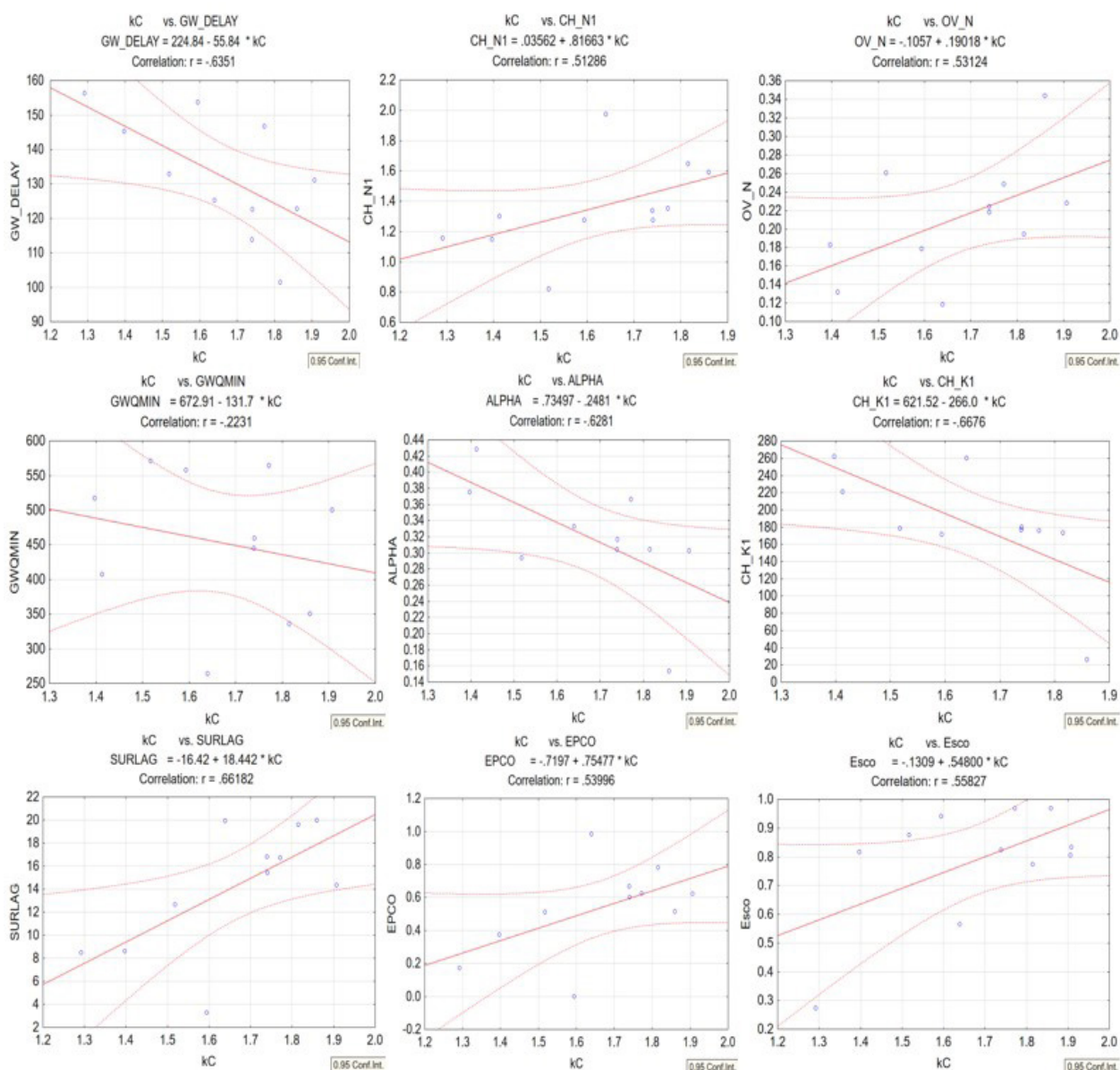


Figure 6. Parameters correlation with the coefficient kC .

2009). The basins that most resemble a circle have smaller values for the parameter Surlag, as found in the correlation with the kC.

The parameters Ch_k1, Gw_delay and Alpha_bf have a correlation inversely proportional to kC. Therefore, the more the basin shape resembles a circle, the greater the parameter values. The function of parameter Ch_k1 is to control surface transmission losses as it flows into the main channel (ARNOLD et al., 2012). Thus, in circular basins the water loss increases due to the flow tending to accumulate in the exudate (OLIVEIRA et al., 2007). The parameter gw_delay has the function of delaying groundwater flow, stipulating the percolation time. It mainly depends upon the water table depth and hydraulic properties. The parameter Alpha_bf is a recession constant, which indicates the response of the groundwater flow to changes in recharge (ARNOLD et al., 2012). These two parameters are correlated because elongated basins have a longer drainage time, requiring smaller values for compensation.

The parameters Ov_n and Ch_N1 are the flow roughness coefficients (Manning coefficient) and can be defined as the friction the water suffers during the flow (MATOS et al., 2011). Comparing them with the kC, a direct relationship is observed, which could be caused by the decrease in concentration time in circular basins, resulting in the Manning coefficient decrease (SILVEIRA, 2005).

The parameter Esco is a compensation factor for soil evaporation. The model is able to draw more water from the lower levels due to the reduced value of this parameter (ARNOLD et al., 2012). This justifies the direct relationship between the ESCO parameter and kC, as circular basins tend to have a shorter concentration time, generating less water storage.

The application of regression equations developed from the comparison between calibration parameters and coefficient kC was carried out in 22 sub-basins (Table 7) to determine variations

caused by parameters in the water flow in small basins. In choosing the sub-basins, the drainage area did not exceed 110 km² and were contained in most fluvimetric stations' sub-basins used in the model calibration (Figure 1). The coefficient kC of each sub-basin was also compared with the coefficient kC of the corresponding fluvimetric station sub-basin.

The variations in the estimated average flows comprised between 0.8 and 42.1%. Therefore, depending on how the sub-basin calibration occurred, mean flow values can be underestimated or overestimated by up to 42.1%.

The difference between the simulated Q95 initially and after applying the regression equations ranged from 7.7 to 82.7%. This indicates the parameters related to basin shape have a great influence on the minimum simulated flow. Therefore, it is necessary to compensate these values in order to manage water resources in the sub-basins.

The results corroborated the study by Spruill, Workman and Taraba (2000). They found the most influential flow calibration parameters in small basins were related to hydraulic conductivity, channel base flow, drainage area, channel length and width. Thus, the uncertainty in the small-scale water flow representation is mainly associated with basin shape, and the coefficient kC can be used to decrease the variable uncertainty. Circular basins tend to have faster response time than elongated basins, so water flow varies greatly when the calibrated basin shape differs from that analysed sub-basin.

In order to emphasize the results, the mean flow variation and the Q95 with parameters was used in the model calibration. The drainage area and the coefficient kC (Table 8) were compared using the Pearson coefficient. The Pearson coefficient must have

Table 7. Difference between flow simulations in sub-basins from 2010 to 2016.

Sub-basin area km ²	Sub-basin area of the fluvimetric station km ²	kC sub-basin	kC sub-basin of the fluvimetric station	ΔkC (%)	$\Delta Q_{Average}$ (%)	$\Delta Q_{95\%}$ (%)
17.12	11,803	1.93	1.76	17.3	4.7	8.1
8.19	11,803	2.04	1.76	27.5	1.2	82.7
10.23	11,803	1.7	1.76	6.3	3.7	7.7
9.36	11,803	1.64	1.76	11.8	7.7	47.3
17.12	9,070	1.93	1.74	19.4	11.2	14.6
6.61	9,070	1.82	1.74	7.8	5.7	15.9
8.83	9,070	1.67	1.74	6.5	1.4	7.8
22.67	5,160	1.75	1.81	6.4	6.4	22.7
18.66	5,160	2.15	1.81	33.2	3.2	51.4
9.43	5,160	1.9	1.81	8.9	4.6	13.2
108.12	3,330	1.76	1.52	24.3	0.8	29.8
14.5	3,330	2.17	1.52	65.4	42.1	59.0
94.57	1,650	1.7	1.86	15.7	3.9	45.8
12.3	1,650	1.97	1.86	11.4	1.3	22.2
5.07	1,650	1.69	1.86	16.5	0.1	48.3
34.89	1,240	2.03	1.91	12.7	8.8	17.7
9.28	1,240	1.9	1.91	0.8	6.8	30.0
8.53	1,240	1.95	1.91	4.6	6.4	30.5
23.38	286	1.99	1.64	34.9	12.7	15.8
10.53	286	1.73	1.64	9.1	16.3	28.1
8.95	286	1.76	1.64	11.9	5.3	17.9

Table 8. Comparison of the Pearson coefficient variation.

	Parâmetros									Area	kC
	ESCO	EPCO	SURLAG	GW_DELAY	CH_N1	OV_N	GWQMIN	ALPHA_BF	CH_K1		
ΔQ Average	0.32	0.44	0.45	0.3	0.37	-0.02	0.35	0.15	0.62	-0.18	0.68
ΔQ Q95	0.04	0.04	0.5	0.29	0.2	0.17	0.29	0.6	0.48	0.37	0.5

values equal to or greater than 0.5. Thus, there is a correlation between the parameters compared.

The variations between Q95 and the Surlag, Alpha_Bf and kC coefficients are correlated, indicating the changes in recharge and the daily surface runoff discharged into the main channel are influential factors in flows with 95% occurrence frequency. It also correlated the average flow variations with the coefficients Ch_k1 and kC, demonstrating the importance of surface transmission losses in the average flow determination.

Another point of uncertainty is the study area discretisation. When it represents small basins, one must detail the soil structure, the land use as well as the land slope. In this way, the model can better simulate the water flow, sediments and nutrients. Jha et al. (2004) also presented this finding. They noted the sub-basin division depends on the expected model response. In this case, to control the uncertainties, it is necessary to plan basin division and discretisation based on the expected hydrological simulation responses.

CONCLUSION

The HRUs discretisation has shown there is no need to discriminate the sub-basins when the goal is to represent the flow peaks in a small basin. However, if the objective is to verify the base flows, the number of HRUs is fundamental to a good hydrological processes representation.

According to the results obtained by applying the regression equations drawn between the comparison of the calibration parameters and the coefficient kC, it was determined the basin shape coefficient can generate up to 42.1% of variation in mean flow and 82.7% in Q95 of the sub-basins.

Thus, the regression equations used can help reduce uncertainties generated during parameter calibration. When using the model as a tool for water resources management, it is necessary to represent the hydrological processes' spatial variabilities of all the sub-basins involved in the study area.

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Pedro Thiago Venzon: Contributed with the elaboration of the study central idea. Carried out the hydrological simulations, analysis and discussion of the results, preparation of the manuscript including writing, drawing of figures and tables and bibliographic review.

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