

RESEARCHING NEW WAYS TO REDUCE N₂O EMISSION FROM A GRANULAR SLUDGE SEQUENCING BATCH REACTOR TREATING DOMESTIC WASTEWATER UNDER SUBTROPICAL CLIMATE CONDITIONS

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Abstract - N₂O emissions from wastewater treatment plants have become an important issue, since this compound is a significant greenhouse gas that affects the sustainability of sewage treatment. This work aimed to investigate and to reduce N₂O emission from a pilot-scale aerobic granular sludge sequencing batch reactor (AGS-SBR) operated for carbon and nitrogen removal from domestic wastewater under subtropical climate condition. Three operational strategies (S-I, S-II and S-III) with different anoxic phase durations were compared regarding treatment efficiency and N₂O emission. For all the studied strategies, volatile suspended solids (VSS) was between 1.0 and 1.2 g/L. S-III, with the longest anoxic phase, obtained the highest biological oxygen demand (BOD) and NH₄⁺-N removal efficiencies (86% and 84%, respectively), the lowest N₂O emission factor (16.99 gN₂O-N/person·year) and the lowest total nitrogen (TN) to N₂O conversion ratio (0.47%). The results indicated that the extension of the anoxic phase was an effective way to significantly reduce N₂O emission and to improve treatment efficiency.

Keywords: Nitrous oxide; Aerobic granular sludge; Domestic wastewater; Sequencing batch reactor; Subtropical climate.

INTRODUCTION

Production and emission of greenhouse gases (GHG) from wastewater treatment plants (WWTP) is a very important issue, which is becoming increasingly significant (Foley et al., 2010; Mannina and Cosenza, 2015). In particular, N₂O emission in wastewater treatment systems with biological nutrient removal deserves special attention, since this compound is one of the main GHG, and its global warming potential is about 300 times higher than that of CO₂ (Yang et al., 2013).

For over a century, the conventional activated sludge process has been a standard model of biological wastewater treatment systems. However, this technique has as main drawbacks the poor settling properties of the biomass, which is in the form of flocs and can compromise the quality of the final effluent, and the requirement of large areas to place the secondary clarifiers (Liu and Tay, 2004). Sequencing batch reactors (SBR) with aerobic granular sludge (AGS) are presented as a promising option for the biological treatment of domestic and industrial effluents, due to

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the efficiency and robustness of this type of system (Di Bella and Torregrossa, 2013; Moreira et al., 2015; Pronk et al., 2015). With this technique, it is possible to obtain high removal rates of organic matter and nutrients in a single reactor, producing a final effluent with high quality (De Kreuk et al., 2005; Liu et al., 2015). However, the dynamics of nitrogen removal in this process, including nitrous oxide emission, are not completely understood; thus, further research is required.

In the current scientific literature, there are few studies reporting N_2O emission by biological treatment systems using aerobic granular sludge. The few studies that address this topic are generally related to systems fed with synthetic wastewater (Rathnayake et al., 2013; Wei et al., 2014), being even more scarce reports concerning the emission of N_2O by granular sludge fed with real wastewater (Castro-Barros et al., 2015), especially under tropical and subtropical climate conditions.

Wide variations concerning N_2O emissions have been reported in scientific literature, as it can be affected by many factors. Conversions of influent nitrogen to N_2O reported by Kampschreur et al. (2009) ranged from 0.001 to 14.6%. Moreover, a national survey conducted by Ahn et al. (2010) in the United States listed conversions ranging from 0.01 to 1.8% of the influent nitrogen. The N_2O emission factor from a municipal activated sludge WWTP reported by Daelman et al. (2013) was 163.2 g N_2O -N/person·year, which is 80 times higher than the EF proposed by the IPCC (2006). Thus the importance to estimate a N_2O emission factor for each specific condition.

With this work, the aim was to advance the knowledge concerning the behavior and the rates of N_2O emission in a pilot-scale SBR with AGS fed with domestic wastewater under subtropical climate conditions. The objectives of this research were: i) to develop aerobic granular sludge in a pilot SBR fed with municipal sanitary wastewater; ii) to evaluate the performance of the treatment; and iii) to monitor, to quantify and to compare N_2O emissions from the SBR operated under three cycle configurations.

MATERIALS AND METHODS

Pilot reactor operation and cycle configurations

The pilot reactor studied was an acrylic bubble column 2.18 meters high and with a 25 cm internal diameter, which worked as a SBR. The working volume was 98 L and the reactor outlet pipe provided the exchange of 56% of the liquid volume, in order to treat 55 L of influent during each cycle. The reactor was operated at room temperature (21–26°C) without pH control.

The reactor was operated under three different strategies: Strategy I (S-I), Strategy II (S-II), and Strategy III (S-III), each one with different anoxic phase duration. The cycle phases configuration of the three strategies are shown in Figure 1. During the anoxic phase of S-III, in order to maintain the sludge dispersed through the liquid column when aeration was off, there were aeration pulses of 10 seconds each 15 minutes, which were short enough to keep a very low dissolved oxygen (DO) concentration in the mixed liquor. The variation of the anoxic and aeration

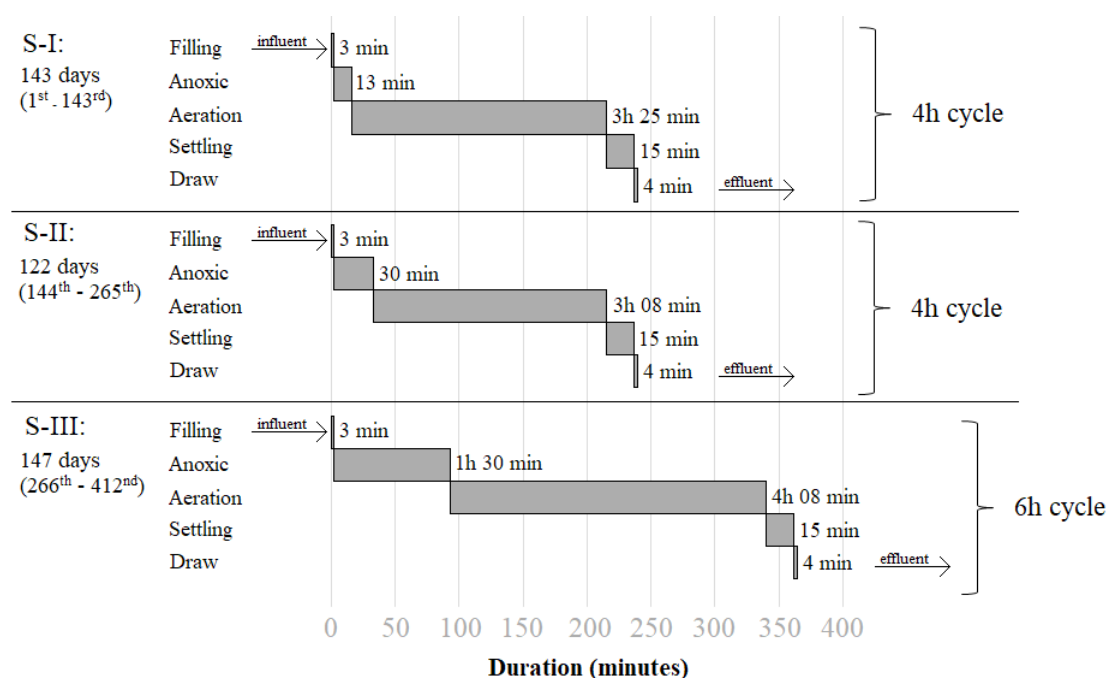


Figure 1. SBR cycle phases configuration during S-I, S-II and S-III.

Table 1. Characteristics of the domestic wastewater used to feed the system.

Parameter	Unit	S-I	S-II	S-III
pH	-	6.64 ± 0.44	7.12 ± 0.10	6.98 ± 0.07
Temperature	°C	21.6 ± 2.2	23.1 ± 2.0	26.2 ± 1.3
COD _{total}	mg/L	518 ± 95	498 ± 187	400 ± 101
COD _{soluble}	mg/L	252 ± 40	176 ± 43	186 ± 32
BOD _{5, 20}	mg/L	334 ± 60	290 ± 95	221 ± 36
NH ₄ ⁺ -N	mg/L	52 ± 9	47 ± 9	49 ± 10
TN	mg/L	62 ± 6	58 ± 5	61 ± 7
Organic loading rate (total)	kg COD/m ³ ·d	1.76 ± 0.32	1.69 ± 0.64	0.91 ± 0.23
Organic loading rate (soluble)	kg COD _s /m ³ ·d	0.86 ± 0.14	0.60 ± 0.14	0.40 ± 0.07
Ammonium loading rate	kg NH ₄ ⁺ -N/m ³ ·d	0.17 ± 0.03	0.16 ± 0.03	0.11 ± 0.02

phase durations was tested to verify its influence on the treatment efficiency and on the N₂O emission. The SBR cycle configuration corresponded to a hydraulic retention time (HRT) of 7.13 hours for S-I and S-II, and 10.69 hours for S-III.

These 3 cycle configurations were adopted in order to favor granular sludge formation and accumulation in the reactor, and to compare the SBR performance under different regimes. The intention was to obtain aerobic granules with a gradient of oxygen through them, providing aerobic/anoxic layers and the occurrence of simultaneous nitrification-denitrification (SND).

In this study, real domestic wastewater was used to feed the system (influent characterization is in Table 1). The wastewater was collected from an inspection chamber of the public sewage system, located in the city of Florianópolis, state of Santa Catarina, south of Brazil (27°36'12.7"S; 48°31'14.9"W). During the SBR start-up period, wash-out conditions were applied to favor the aerobic granules formation. The selective conditions applied were the shear force caused by the aeration, maintained at 32 L_{air}/min, and the reduced settling time (15 minutes). The settling velocity imposed to the biomass to be retained inside the system was 7.47 cm/min.

Analytical procedures

The sludge settleability was assessed through sludge volume index (SVI), which was calculated at times of 5, 10 and 30 minutes (SVI₅, SVI₁₀ and SVI₃₀, respectively) (Schwarzenbeck et al., 2004). Granulometric analyses were performed through laser diffraction (Mastersizer 2000 – Malvern Instruments, UK), and optical microscopy (Olympus BX40, Japan) was used to observe granules formation. According to Liu et al. (2010), the sludge is considered granular when at least 50% of the biomass presents diameter over 200 μm.

The performance of the wastewater treatment was analyzed in terms of carbon and nitrogen removal. Biochemical oxygen demand (BOD), chemical oxygen demand (COD) and total suspended solids (TSS) were quantified according to Standard Methods (APHA, 2005). Nitrite and nitrate were quantified by ion

chromatography (DIONEX ICS-500). Total nitrogen (TN) and ammonium (N-NH₄⁺) were quantified using Hach® test kits (High Range Nitrogen-Ammonia Reagent Set: #2606945; High Range Total Nitrogen Reagent Set: #2714100). A multiparameter probe (YSI 6820, USA) was used to measure dissolved oxygen (DO) concentration regularly.

To perform N₂O measurements, the upper part of the reactor was sealed with an airtight lid, which would only allow the air flow through a PVC hose connected to it. The airflow was continuously monitored by a gas analyzer (Guardian SP - Edinburgh) based on dual wavelength infrared sensor technology.

N₂O emission quantification

For purposes of quantifying N₂O emission, a graph of N₂O concentration versus time was plotted, and the area under the curve was calculated using a definite integral. The total amount of N₂O released in one SBR cycle was calculated according to Eq. 1.

$$m_{N_2O} = \int (C_{N_2O} \times dt) \times Q_{air} \times \frac{1}{60} \quad (1)$$

where m_{N_2O} is the total amount of N₂O released in one SBR cycle (g N₂O); $\int(C_{N_2O} \times dt)$ is the integral of the variation of N₂O concentration over time (g N₂O·s/L_{air}); Q_{air} is the air flow rate (L_{air}/min).

The total amount of nitrogen released to the atmosphere in a SBR cycle was calculated according to Eq. 2.

$$m_{Nd} = (TN_I - TN_E) \times V_{tc} \times \frac{1}{1000} \quad (2)$$

where m_{Nd} is the amount of released nitrogen in one SBR cycle (g N); TN_I the total nitrogen influent concentration (mg N/L); TN_E the total nitrogen effluent (mg N/L); V_{tc} is the volume of wastewater treated in one SBR cycle (L).

The conversion of total influent nitrogen to N₂O was calculated by relating the amount of N₂O-N released in a SBR cycle, the TN influent concentration and the volume of wastewater treated in a SBR cycle (Eq. 3).

$$\text{Conversion}_{N_{\text{inf}} \rightarrow N_{2\text{O-N}}} (\%) = \frac{m_{N_{2\text{O-N}}}}{\text{TN}_1 \times \frac{1}{1000} \times V_{\text{te}}} \times 100 \quad (3)$$

where $\text{Conversion}_{N_{\text{inf}} \rightarrow N_{2\text{O-N}}}$ is the fraction of influent nitrogen converted to N_2O (%); $m_{N_{2\text{O-N}}}$ is the amount of N_2O released in a SBR cycle, expressed in terms of nitrogen (g N_2O-N).

The N_2O Emission Factor (EF) was calculated by relating the amount of released N_2O , the daily per capita wastewater generation and the volume of wastewater treated in one SBR cycle, according to Eq. 4.

$$\text{EF} = m_{N_{2\text{O-N}}} \times \frac{Q_{\text{pc}}}{V_{\text{te}}} \times 365 \quad (4)$$

where EF is the N_2O Emission Factor (g N_2O-N /person-year); $m_{N_{2\text{O-N}}}$ is the amount of released N_2O , expressed in terms of nitrogen (g N_2O-N); Q_{pc} is the daily per capita wastewater generation (L/person-d).

The N_2O Flow-Based Emission Factor (FBEF) was calculated by relating the amount of released N_2O and the volume of wastewater treated in one SBR cycle, according to Eq. 5.

$$\text{FBEF} = \frac{m_{N_{2\text{O-N}}}}{V_{\text{te}}} \quad (5)$$

where FBEF is the N_2O Flow-Based Emission Factor (g N_2O-N/L).

Microbiological procedures

Next-generation sequencing and fluorescent *in situ* hybridization (FISH) techniques (van Loosdrecht et al., 2016) were used to investigate the microbial community of the granular sludge samples from S-I, S-II and S-III. DNA sequencing was performed using MiSeq[®] Illumina technology for sequencing by synthesis (SBS). The DNA was extracted from biomass samples using a MoBio PowerBiofilm[™] DNA extraction kit (MoBio Laboratories, USA). The rRNA 16S V3/V4 region was amplified using the 341F (CCTACGGGRSGCAGCAG) and 806R (GGACTACHVGGGTWTCTAAT) primers, with Illumina adapters, required for sequencing. The amplification was performed in 35 cycles at 50°C of annealing temperature, where each sample was amplified in triplicate. The sequencing was performed in Illumina MiSeq[®], using the V2 kit, with a single-end 300 runs. The system guaranteed the reading of 100,000 sequences with sampling taxonomic identification and quantification of the number of sequences obtained from each taxon. OTU Picking was performed using

BLASTN 2.2.28 against GreenGenes 13.8 database. To attribute taxonomy, only sequences with hits higher than 99% of identity in alignment were considered.

The fluorescent *in situ* hybridization (FISH) technique was used to investigate the microbial community of the granular sludge samples from S-I, S-II and S-III. Granular sludges sampled from the reactor were fixed in 4% paraformaldehyde solution for 2–3 h at 4°C. The sludge samples were rinsed twice with phosphate-buffered saline (PBS) and then dehydrated by successive 50%, 80%, and 98% ethanol washes (Amann, 1995). *In situ* hybridizations were performed using the specific probes NSO190, Ntspa662 and PAE997 for ammonium-oxidizing bacteria (AOB), nitrite-oxidizing bacteria (NOB) and *Pseudomonas* genus, respectively. Oligonucleotides were synthesized and fluorescently labeled with a hydrophilic sulfoindocyanine dye (Cy-3) at the 5' end. Details on oligonucleotide probes are available at probeBase.

RESULTS AND DISCUSSION

Biomass characteristics and composition

AGS was successfully cultivated in the SBR fed with domestic wastewater containing low concentration of organic substrate, without adding an external carbon source and without biomass inoculation. The average biomass concentrations in the mixed liquor during S-I, S-II and S-III were 1100 mg/L, 1200 mg/L, and 1050 mg/L, respectively. Considering granules to be the particles with diameters between 0.2 and 5.0 mm (Liu et al., 2010), granulometric analyses indicated average granular biomass fractions of 66%, 32% and 59% during S-I, S-II and S-III, respectively.

Initially, before the granules formation, all particles presented diameters below 200 μm . On the 16th day, 83% of the biomass reached diameters higher than 200 μm , and 46% of the particles were greater than 600 μm . The granular biomass varied throughout the studied strategies, showing mean diameters of 427 ± 89 , 265 ± 51 and 292 ± 54 μm for S-I, S-II and S-III, respectively.

According to de Kreuk et al. (2007), the biomass is considered to be predominantly granular when at least 50% of the biological aggregates present diameters superior to 200 μm . Therefore, the sludge was considered predominantly granular during S-I and S-III. However, the same authors noted that other characteristics, such as SVI, must also be considered in granular systems evaluations.

During S-I, $\text{SVI}_{5^{\circ}}$, SVI_{10} and SVI_{30} presented the highest variation. This fact can be attributed to biomass instability during the granules formation and stabilization, as granulation is a gradual process comprising three stages: (i) sludge acclimation, (ii)

sludge aggregation and (iii) granules maturation (Wang et al., 2005). Low SVI values and SVI₃₀/SVI₁₀ ratios close to 1 are associated with a denser and more compact biomass, with good settling properties. The mean SVI₃₀ values were 126, 118 and 70 mL/g for S-I, S-II and S-III, respectively. In another study with AGS in SBR operated under similar conditions, Wagner and Costa (2013) verified that SVI₃₀ decreased gradually and stabilized at 53 mL/g after 100 days of operation. During S-II, SVI₅, SVI₁₀ and SVI₃₀ values were closer than under S-I, indicating an improvement in the settleability. The SVI₃₀/SVI₁₀ ratio remained 0.8 from the 149th until the 230th day, indicating an improvement in granular structural stability and biomass compactness, even with biomass concentration variations in the reactor.

S-III, which presented the longest anoxic phase, showed the closest values among SVI, and the average SVI₃₀/SVI₁₀ ratio was 0.88±0.09, reaching 1.0 on the 286th and 356th days. According to de Kreuk et al. (2007), the SVI₃₀/SVI₁₀ ratio gives excellent information regarding the granular fraction of the biomass. The higher the ratio, the better the granule settleability and compactness. Furthermore, the SVI₃₀/SVI₁₀ ratio also indicates the granulation process status (Liu and Tay, 2007). These authors consider the granulation process to be completed when the SVI₃₀/SVI₁₀ ratio reaches 0.9. Therefore, even with the diameter decrease verified in S-II and S-III, the granule settleability and compactness improved. According to Liu and Tay (2007), a higher granule size does not guarantee a better settleability, while the SVI is directly related to sludge density. This means that aerobic granulation should not be restricted only to the granule size increase, but also to the improvement in the biomass compactness and settleability.

Wastewater treatment performances

The effluent concentrations and the removal efficiency in terms of carbon and nitrogen verified for S-I, S-II and S-III are presented in Table 2. Although there were fluctuations in the soluble COD influent, the effluent concentrations did not show considerable variations. Under S-I, the COD_{soluble} removal efficiency was 79%, with a mean effluent concentration of

52 mg/L. During S-II, the removal efficiency was 70%, with the SBR effluent presenting an average concentration of 50 mg/L. In S-III, the COD_{soluble} removal rate was 68% and the effluent concentration was 58 mg/L.

Organic matter removal was also analyzed through BOD concentration. As observed with COD, influent BOD varied since the system was fed with real wastewater. An improvement in the BOD removal was observed over time, from 69 to 86%, and effluent concentrations from 106 to 31 mg/L. Regarding Brazilian national regulations CONAMA 430/2011, which require at least 60% removal or an effluent BOD concentration below 120 mg/L, all strategies met the quality criteria for carbon removal. The BOD removal improvement that was observed in S-III might also be related to the change in the HRT, which went from 7.13h (S-I and S-II) to 10.69h (S-III).

In terms of nitrogen removal, under S-I and S-II the NH₄⁺-N average removal efficiency was below 60%. The longest aeration phase, applied in S-III, favored ammonium removal, achieving a stable and effective removal of 80% under this operational condition. In fact, as can be seen in Figure 2, which shows the pH profiles during the GSB cycles, S-III presented the highest decrease in pH during the aerobic phase, indicating the occurrence of a more intense nitrification process in relation to S-I and S-II. As with the BOD removal, the NH₄⁺-N removal improved with the increase in the HRT from S-I and S-II to S-III. This fact is consistent with the results presented by Wagner and Costa (2013), who observed a significant increase in the NH₄⁺-N removal when the HRT went from 7.5h to 10h, when operating a SBR under conditions similar to the present study.

During aeration, due to nitrification, there was a progressive increase of nitrite formation, with mean effluent concentrations of 5.9 (S-I), 9.7 (S-II) and 14.5 (S-III) mg NO₂⁻-N/L. Nitrate was formed in trace concentrations in S-I and S-II, remaining at low levels during these operational strategies, in a range between 0.1 and 0.4 mg NO₃⁻-N/L. During S-III, a higher nitrate formation was observed, with a mean effluent concentration of 4.15±1.29. These results indicate the occurrence of incomplete nitrification in all conditions

Table 2. Effluent concentrations and removal efficiencies verified for S-I, S-II and S-III.

	Strategy I		Strategy II		Strategy III	
	Duration: 143 days		Duration: 122 days		Duration: 147 days	
	Effluent (mg/L)	Removal efficiency (%)	Effluent (mg/L)	Removal efficiency (%)	Effluent (mg/L)	Removal efficiency (%)
COD _{total}	150±63	69±13	89±34	79±14	136±23	64±6
COD _{soluble}	52±12	79±5	50±12	70±12	58±9	68±4
BOD	106±31	69±6	70±28	76±7	31±7	86±3
NH ₄ ⁺ -N	24±11	52±15	20±11	57±18	8±4	84±8
NO ₂ ⁻ -N	5.9±2.5	-	9.7±3.7	-	14.5±5.7	-
NO ₃ ⁻ -N	0.14±0.05	-	0.30±0.14	-	4.15±1.29	-

tested, with higher nitrite accumulation and nitrate formation at the longest aeration period. The extension of the anoxic phase of the cycle did not show as great an influence on nitrite accumulation as the extension of aeration phase.

Nitrite accumulation in reactors with aerobic granules has been reported by some authors (Yang et al., 2013; Isanta et al., 2012; Coma et al., 2012), including conditions of low-strength wastewater (Wang et al., 2007; Figueroa et al., 2008). Although nitrite-oxidizing bacteria (NOB) have their activity decreased by low DO concentration, in the present study the cause of partial nitrification was not low DO in the mixed liquor, since DO remained close to 8 mg/L throughout the aeration phase of the operational cycle, due to the high level of aeration required. Typical pH and DO cycle profiles can be seen in Figure 2.

In this study, the temperature might have been one important factor that contributed to nitrite accumulation, since it reached values up to 27° C inside the reactor. The maximum specific growth rate

of ammonium-oxidizing bacteria (AOB) is higher than that of NOB at temperatures above 15° C (Bérnet and Spérandio, 2009), which can favor nitrite accumulation. In fact, this is the basis of SHARON technology, which consists of a chemostat reactor operated at 30°C with a low HRT, to favor AOB growth and NOB washout, achieving partial nitrification (Hellings et al., 1998). The present research showed that, when the HRT increased from 7.13h (S-I and S-II) to 10.69h (S-III), there was a higher nitrate production, i.e., a more complete nitrification process.

The solids retention time (SRT) is another parameter that can influence nitrite accumulation. The calculated SRT were 14, 15 and 9 days for S-I, S-II and S-III, respectively. However, in the present research, nitrite accumulation might have been more strongly associated with the cycle duration than with the SRT. The cycle duration, which was extended from 4h (S-I and S-II) to 6h (S-III), favored the occurrence of nitrification, as there was a longer time to allow ammonium oxidation.

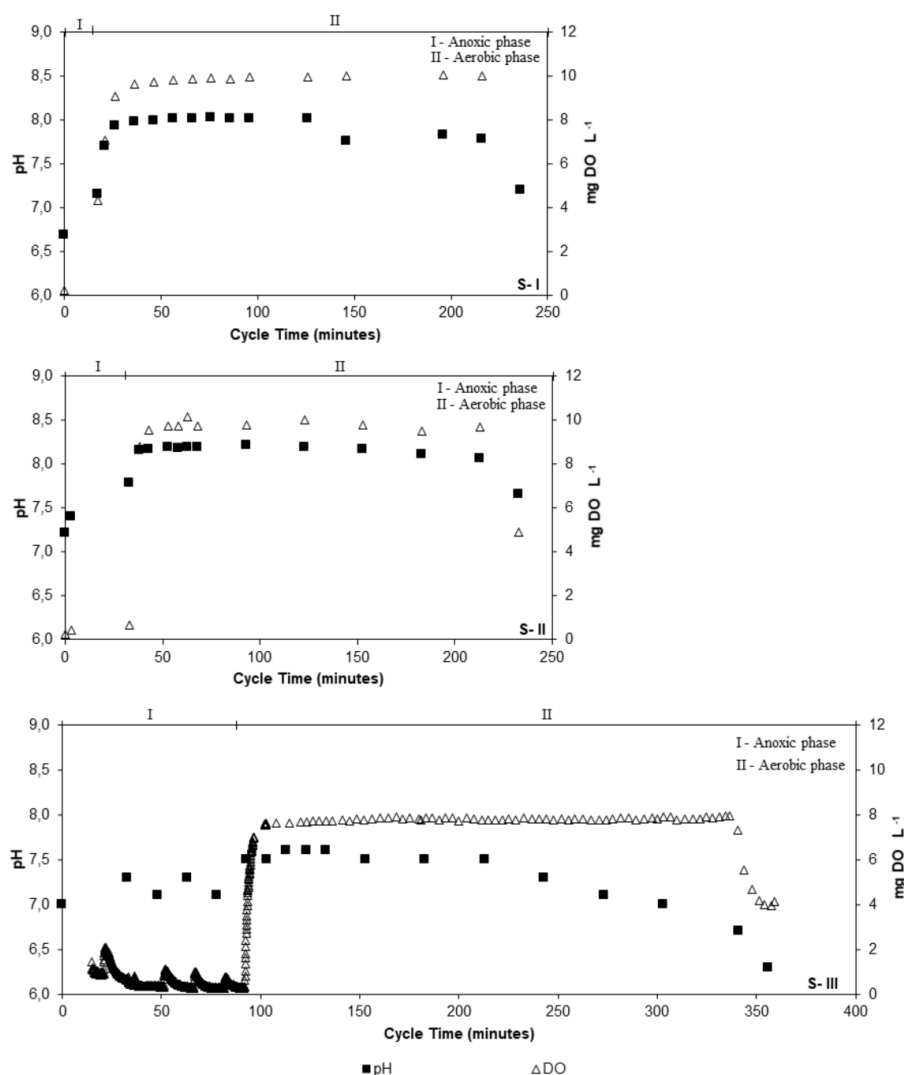


Figure 2. Profiles of pH and DO concentration during a typical SBR cycle of each strategy (S-I, S-II and S-III).

Besides temperature and cycle duration, another factor that could contribute to nitrite accumulation is AOB and NOB stratification and the existence of an oxygen gradient in the aerobic granules. The presence and predominance of AOB colonies in the outer layer of the granules could favor nitrification, since they are in a more beneficial position for oxygen consumption than NOB, which are present in the inner layers (Poot et al., 2016; Guimarães et al., 2017). Many studies have tried to achieve partial nitrification, while this study showed feasible SBR operational conditions in tropical and subtropical climate for this.

The nitrite route has several advantages, including a lower oxygen requirement for nitrification (25% less), lower organic carbon consumption in denitrification (40% less) and lower sludge production (Van Loosdrecht and Jetten, 1998). These advantages are even more notable in the case of nitrogen-rich wastewater with a low organic carbon content. However, one of the main concerns related to nitrite accumulation is N₂O production. Several studies have shown that nitrite accumulation is usually accompanied by higher N₂O emissions (Itokawa et al., 2001; Kampschreur et al., 2008).

N₂O emissions

The variation of N₂O emitted in one typical cycle of the SBR in each operational condition tested is shown in Figure 3. For all of the studied strategies, N₂O emission was not constant during the cycle phases. During S-I and S-II, the emission of N₂O started at the beginning of the aerobic phase, and the peak concentration of N₂O occurred in the early moments of aeration, between 2' and 2'30", and then decreased until it ceased. During S-III, the N₂O emission pattern was similar to S-I and S-II. However, some N₂O emission was also observed during the aeration pulses applied in the anoxic phase of this strategy. The maximum emissions were 0.90, 0.36 and 0.12 mg N₂O/s for S-I, S-II and S-III, respectively, showing that, when extending the anoxic phase, a lower N₂O emission peak was observed.

The fact that the peak emission of N₂O occurred at the beginning of the aerobic phase does not mean that this is the moment of greatest N₂O production. In fact, the emission pattern suggests that denitrification was possibly the major source of N₂O generation, probably due to the occurrence of an incomplete denitrification process. Moreover, there seems to be no N₂O formation during the occurrence of nitrification. Yang et al. (2013), who investigated N₂O emission by a single stage reactor with partial nitrification/anammox, suggested that N₂O emitted during aeration is produced by microorganisms during the anoxic phase of the reactor cycle. Since there is no air flow during the anoxic phase, N₂O is retained in the system

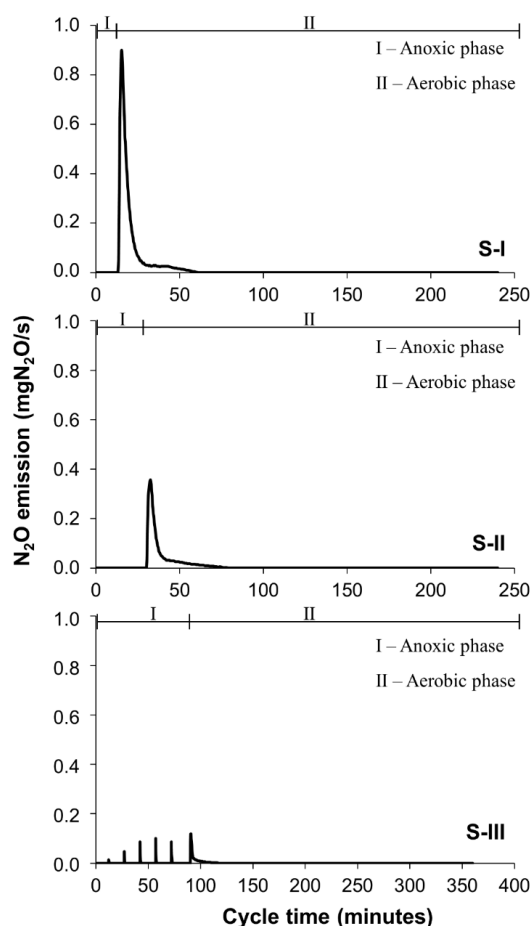


Figure 3. Variation of N₂O emitted during a typical SBR cycle of each strategy (S-I, S-II and S-III). (I) Anoxic phases, (II) Aeration phases.

and accumulates during this step. Mello et al. (2013) investigated the emission of N₂O by an activated sludge treatment plant with intermittent aeration and found that less than 1% of the produced N₂O was released in the absence of aeration.

Therefore, the N₂O that accumulated in the mixed liquor during the anoxic phase is released when aeration starts, causing a peak in the concentration of N₂O emitted at the beginning of the aerobic phase. After a few minutes, all the accumulated N₂O ends up being released to the atmosphere and the emission falls to values near zero.

There are several factors that suppress N₂O production, and consequently the N₂O emission, under the aeration phase of the reactor cycle. Kampschreur et al. (2008) found that high concentration of DO during nitrification prevented N₂O production by microorganisms. In the present study, DO values during the aerobic phase were between 8.0 and 8.8 mg/L, which could prevent the formation of N₂O in this period.

Conversely, there are aspects that contribute to N₂O emission, such as the occurrence of nitrite accumulation during nitrification, which can later be

converted to N_2O during denitrification. Furthermore, if the denitrification process is incomplete, the N_2O formed might not subsequently be converted to N_2 , causing a higher net N_2O generation. Besides that, incomplete denitrification can lead to a higher nitrite concentration in the mixed liquor. Studies by Shaw et al. (2006) indicated that high concentrations of NO_2^- can positively affect the emission of N_2O in a nitrifier denitrification process, making NO_2^- concentration one of the important variables that could be related to N_2O generation during denitrification in activated sludge systems.

Some N_2O emission parameters verified in this research are shown in Table 3. The average N_2O emission was 0.181, 0.058 and 0.016 $gN_2O-N/cycle$ for S-I, S-II and S-III, respectively. In terms of volume of wastewater treated, the flow-based emission factors observed were $3.29 \cdot 10^{-3}$, $1.05 \cdot 10^{-3}$ and $0.29 \cdot 10^{-3}$ gN_2O/L for S-I, S-II and S-III, respectively. Considering the volume of effluent treated in a cycle, and assuming a per capita wastewater generation of 160 L/person·d, the EF observed during S-I, S-II and S-III were 192.2, 61.6 and 17.0 $gN_2O-N/person \cdot year$. These values are much higher than the guidelines proposed by the IPCC (2006), of 3.2 $gN_2O/person \cdot year$ (i.e., 2.04 $gN_2O-N/person \cdot year$) in the case of wastewater treatment systems with controlled nitrification and denitrification. However, this EF proposed by the IPCC was based on a single experiment of Czepiel et al. (1995), performed in a WWTP in Durham, in northern United States (temperate climate). Therefore, the present study suggests that the N_2O emission by biological treatment systems located in subtropical/tropical regions is likely higher than the emission in temperate regions. Such higher EF values were also observed by Mello et al. (2013) in an intermittent aeration activated sludge system located in the highlands of Rio de Janeiro, Brazil, which is a subtropical region.

The total nitrogen removal, the fraction of nitrogen denitrified to N_2O and to N_2 , and the total influent Nitrogen conversion to N_2O are shown in Table 4. The results show that most of the total denitrified nitrogen was converted to N_2 , while only a small fraction was converted to N_2O . Although the complete denitrification process did not occur, N_2 generation predominated over N_2O generation in 84:16, 95:5, and 99:1 ratios for S-I, S-II and S-III, respectively. These results indicate that S-III, with the longest anoxic time, resulted in the lowest N_2O emission among the studied

Table 4. Nitrogen removal, fraction of Nitrogen denitrified to N_2O and to N_2 , and total influent Nitrogen conversion to N_2O .

Strategy	TN removal (%)	Denitrification		Influent N conversion to N_2O (%)
		N_2O (%)	N_2 (%)	
S-I	37.9	15.8	84.2	5.28
S-II	36.2	5.1	94.9	1.95
S-III	44.3	1.1	98.9	0.47

strategies, both in terms of emission factor and TN to N_2O conversion. The ratios of denitrified nitrogen emitted as N_2O observed in this study are within the range of values observed by Foley et al. (2010) in a study involving seven WWTPs in Australia. The authors reported a wide oscillation between the percentages of nitrogen denitrified to N_2O compared to the total nitrogen denitrified, ranging from 0.06 to 25.3%.

The fraction of TN converted to N_2O ranged from 0.47% in S-III to 5.28% in S-I. These values are below the value reported by Sun et al. (2013), who recorded a conversion of total nitrogen to N_2O of 6.52% in a full scale SBR. The same authors reported a conversion of 1.95% of the influent TN to N_2O in a real scale A²O system, this value being within the range observed in the present study. The conversion rate verified in S-II is also very similar to the conversion presented by Castro-Barros et al. (2015), who noted that 2.0 % of the incoming nitrogen load was converted to N_2O .

Kong et al. (2013) analyzed the emission of N_2O by a biofilm SBR under intermittent aeration, with intentional nitrite accumulation to favor the anammox process. The fraction of influent nitrogen converted to N_2O was $1.50 \pm 0.22\%$. This percentage of conversion is very close to what was observed in S-II, in which the occurrence of partial nitrification was also observed. By using molecular biology techniques, the authors found that *Nitrosospira* bacteria were the dominant gender of AOB responsible for N_2O emissions via nitrifier denitrification.

A review study done by Kampschreur et al. (2009) listed conversions of influent nitrogen to N_2O ranging from 0.001 to 14.6%. A wide variation of conversion to N_2O was also found in a national survey conducted by Ahn et al. (2010) in the United States, where conversions ranging from 0.01 to 1.8% of the influent nitrogen were observed. Although there are variations between the values obtained by different authors, it is noted that the total nitrogen fraction converted to N_2O tends to stay

Table 3. N_2O emission parameters for S-I, S-II and S-III.

Strategy	Emission per cycle ($g N_2O-N/cycle$)	Daily emission ($g N_2O-N/d$)	EF ($g N_2O-N/person \cdot year$)	FBEF ($g N_2O-N/L$)
S-I	0.181	1.086	192.19	$3.29 \cdot 10^{-3}$
S-II	0.058	0.348	61.59	$1.05 \cdot 10^{-3}$
S-III	0.016	0.064	16.99	$0.29 \cdot 10^{-3}$

within the range of values between 0.4 and 6.5%, which covers the results obtained in this research.

Although the N₂O EF obtained in this research is superior to the EF proposed by the IPCC (2006), it is consistent with several studies reporting N₂O emissions by activated sludge systems. Daelman et al. (2013), studying the N₂O emission by a municipal activated sludge WWTP, reported an emission factor of 163.2 g N₂O–N/person·year. This EF is 80 times higher than the EF proposed by the IPCC, and even much higher than the EF obtained in S-II and S-III. Mello et al. (2013), investigating the emission of N₂O by an activated sludge WWTP with intermittent aeration, observed an EF of 8.76 g N₂O/person·year, i.e., 5.57 g N₂O–N/person·year. This EF is also higher than the EF proposed by the IPCC (2006), although it is lower than the values observed in the present research.

The average FBEF ranged from $3.29 \cdot 10^{-3}$ gN₂O/L in S-I to $0.29 \cdot 10^{-3}$ gN₂O/L in S-III. These values are much higher than the factor reported by Mello et al. (2013) of $8.0 \cdot 10^{-5}$ g N₂O/L, referring to an activated sludge system with intermittent aeration. However, this study was carried out in a region of humid subtropical climate, located at 600 meters of altitude, during the winter, unlike the conditions of the present study. In addition, the occurrence of nitrite accumulation in the system was not reported by the authors. These conditions may help to explain the low N₂O emission verified by the authors, in relation to the values obtained in the present research.

Castro-Barros et al. (2015) studied the emission of N₂O by a full-scale partial nitrification-anammox granular sludge reactor. The nitrogen load applied to the reactor was 1.75 kg NH₄⁺–N/m³·d, this load being around 10 times higher than the load applied in the present research. The authors verified that the conversion of influent nitrogen to N₂O presented an average value of 2.0%, very similar to the conversion observed in S-II, which corroborates the results verified in the present study. In spite of the wide variation between the applied nitrogen loads, the percentage of nitrogen converted to N₂O was quite similar in both cases.

The nitrite accumulation that occurred in this study, as a result of partial nitrification, may have been one of the factors which favored N₂O production during denitrification (Kampschreur et al., 2009). However, since nitrite accumulation was not directly related to N₂O emission, there might be other factors influencing N₂O emission. As noted by Quan et al. (2012), N₂O emission could also be related to the granule constitution, since the spatial structure of the granules may induce incomplete denitrification, which may also lead to significant N₂O generation.

The results obtained in the present study indicate that S-III, with the longest anoxic phase, promoted

the lowest N₂O emission and the highest ammonium removal rate among the studied strategies, probably due to a higher consumption by a better developed anoxic community. The emission factor and the conversion to N₂O verified in S-III were 11 times lower than in S-I, and 4 times lower than in S-II.

Microbial Communities

The AGS composition was dominated by the genera *Pseudomonas sp.* (17%), *Comamonas sp.* (19%) and *Pseudoxanthomonas sp.* (45%) under S-I, S-II and S-III, respectively. The microbial dynamics characterized by new generation sequencing showed fluctuations along the operational strategies. Under S-I (64th and 83rd days), the most abundant families were Caulobacteraceae, Sphingomonadaceae, Pseudomonadaceae and Rhodocyclaceae. Under S-II (189th day), the family Comamonadaceae was highlighted with relative abundance of 22%, whereas under S-III (293rd and 391st days), the Xanthomonadaceae predominated with 45%. A decrease in the relative abundance of Pseudomonadaceae (genus *Pseudomonas sp.*), as well as an increase of Xanthomonadaceae (genus *Pseudoxanthomonas sp.*) was observed over time during the studied strategies. It is important to point out that both populations are denitrifying. In addition, *Pseudoxanthomonas sp.* is a relevant community for granule structure, since they are EPS producers (Weissbrodt et al., 2014). This result corroborates the better stability of the system in terms of granular biomass characteristics obtained under S-III.

In terms of microorganisms related to the nitrogen cycle, DNA new generation sequencing underestimated *Nitrosomonas* and *Nitrospira* sequences, not detecting them. However, they were identified with FISH analysis. AOB hybridizing to probe Nso190 was identified in low abundance in S-I, which is in accordance with low ammonium oxidation activity at this period. Lower AOB activity could arise from competition with heterotrophic bacteria for oxygen (Okabe et al., 1999). In S-II and S-III, corresponding to the 189th and 293rd days, there was a gradual increase in the abundance of AOB, correlating with a higher nitrification rate. Hybridization with Ntspa662 probes was carried out to identify the presence and distribution of *Nitrospira* in the granular sludge. *Nitrospira* was present as small clusters in low abundance during all strategies. Since nitrite accumulated in the reactor at most times, it was directly available to NOB from the bulk liquid (Kim et al., 2006), but conditions (discussed above) were not completely favorable to their high proliferation. Regarding denitrifiers, a positive hybridization signal was observed for *Pseudomonas sp.* in samples of S-I and S-III, indicating this genus as a feasible community for nitrite denitrification processes. However, the availability of sufficient organic carbon is the key

factor in NO and N₂O consumption activities, as previously reported (Kampschreur et al., 2009). It can be concluded that the longer anoxic phase in the SBR operation cycle (S-III) promoted a BOD removal improvement by denitrifiers such as *Pseudomonas*, ensuring lower emissions of N₂O in the granular sludge reactor under these conditions.

CONCLUSIONS

The treatment performance and the N₂O emission from a pilot-scale SBR with AGS operated under three cycle configurations and fed with domestic wastewater under subtropical climate conditions were monitored and quantified. The nitrification process was incomplete, with nitrite accumulation occurring, which was mainly attributed to the temperature and to the cycle duration. There was a significant reduction of 91% both in the TN to N₂O conversion and in the EF verified for the studied strategies, which were associated with the extension of the SBR cycle anoxic phase and with the higher HRT. Furthermore, the anoxic phase extension and the HRT increase were also associated with higher BOD and ammonium removal rates and with a better biomass stability.

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